CHARLES J. VOLPE, JR PURCHASING AGENT

(203) 797-4571 PHONE (203) 796-1527 FAX c.volpe@ci.danbury.ct.us

March 10, 2015

ADDENDUM #1

To

Bid #01-14-15-05 "Westside Middle School Academy Multipurpose Field Project"

This addendum shall be part of the Purchasing documents for the above captioned Bid. This addendum is to be acknowledged by the bidder by signing as provided below and returning with the bidder's proposal.

SEE ATTACHED: ADDENDUM NO 1, DATED MARCH 9, 2015, 37-PAGES

BID DUE DATE: Remains same – Thursday, March 19, 2015, 10:00 AM

Receipt of the	addendum is hereby acknowledged.
Bidder	
Signature	
Title	
Date	

Charles J. Volpe, Jr. Purchasing Agent City of Danbury March 4, 2015

TO: Prospective Bidders

FROM: JJA Sports

Westside Middle School Academy Multipurpose Field Project Bid No. 01-14-15-05 Project No. 14-28 City of Danbury Public Works Department

This Addendum forms part and modifies request for Proposal Documents dated February 15, 2015 including this Addendum No. 1 dated March 4, 2015. Acknowledge receipt of this Addendum by indicating receipt on the Bid Form under the Addendum Section. (Please sign the attached form and return it with your bid proposal).

Where any original item called for in the Project Manual or indicated on the Drawings is supplemented hereby, the supplemental requirements shall be considered as part of the bid package.

Where any original item is amended, voided, or superseded hereby, the associated provisions of such items not specifically amended, voided or superseded shall remain in full effect.

Bid Conditions Division 01

1. Table of Contents

The Table of Contents is revised to add the following:

"Appendix:

Geotechnical Exploration and Report

Dr. Clarence Welti, P.E. P.C.

Geotechnical Study for Proposed Synthetic Field, Light Poles and Scoreboard Mill Ridge Schools, Danbury, CT., December 10, 2014"

Document consisting of 18 pages is attached as part of this Addenda

2. Bid Form:

The Bid Form is revised to include Unit Price Item No 8. The Bid Form consisting 8 pages is attached.

3. Section 01 12 00 Special Conditions:

Revise Section 01 12 00 Special Conditions 1.03 SCHEDULE AND COMPLETION to <u>add</u> the following:

E. Sport light and scoreboard footings, and scoreboard columns shall be completed by; June 12, 2015

F. Sport light poles and Scoreboard shall be completed by; July 3, 2015

Project No. 2014-083A Print Date: 3/10/2015

- G. All work within the field area (including installation of the resilient pad) and parking lot binder shall be completed by; July 3, 2015
- H. We anticipate that the field installation is completed by July 31, 2015
- I. Finish pavement and all remaining work shall be completed by August 7, 2015 Note finish pavement must be installed after completion of the synthetic turf sport field.
- 4. Section 01 22 00 Unit Price

Revise the schedule of Unit Prices to include item No 8 as follows:

- 8. Provide a standard 36" round transformer concrete vault (see attached) with a top 3'x3' hole; CL&P will install a switch and enclosure. The location of the vault shall be approximately 3' to 5' from the existing CL&P service pole. Final location shall be determined by CL&P. The contractor shall install the vault, schedule 40 4" conduit and sweep onto the pole. 4" schedule 40 conduit from the switch vault to the transformer vault entering on the left side. Schedule 40 4" conduit then will have to be run from the right side of the transformer vault to the meter pedestal location. This Unit Price shall include three additional steel bollards. The unit price shall include all excavations, trenching, concrete, backfilling, grading, topsoil and reseeding. The unit price shall include all overhead, profit, fees, insurance, and bond costs.
- 5. Section 01 70 60 Field Engineering

Clarification: The project requires a registered Professional Surveyor for all layout and asbuilt purposes.

Technical

1. Section 11 68 33 Athletic Equipment, 2.7 NETTING SYSTEM

Replace paragraph 2.7, B with the following:

B. Netting panels for the south side ball control netting system heights shall be factory assembled to match the required 15 foot top height. Extent of netting is identified on the drawings and have a total finish height of 15 feet.

Netting panels for the parking lot side ball control netting shall system heights shall be factory assembled to match the required 16.5 foot height. Extent of netting is identified on the drawings and has a total finish height above the top rail of the fence and overhang 3" below the top rail fence of 16.25 feet. This netting shall site above the 45" chain link fence and extend to a height that is 20 feet above the field level.

Replace paragraph 2.7, D, Netting Posts, 1, with the following:

- 1. Net posts for the south side ball control netting system shall include ground sleeves and be 15.5 feet in exposed height above finish grade. Ground sleeves shall be 4.30in OD (4.10in ID) with a depth of 48 inches and imbedded in concrete per manufacturer's requirements. End posts footings shall be sized to accommodate larger loading per end of cable conditions.
- 2. Net posts for the parking lot side ball control netting system shall be integral with the chain link fencing system and shall alternate with intermediate fence posts. Posts shall be galvanized steel

Project No. 2014-083A Westside Middle School Academy Multipurpose Field
Print Date: 3/10/2015 DANBURY, CT

painted posts with one coat of primer and two finish coats of black paint Colorgaly or equal. Sized of posts shall be as depict in the drawings.

2. Section 31 10 00 Site Preparation:

Section 31 10 00 Site Preparation, 3.03 is replaced in its entirety with the Following:

3.03 REMOVAL OF NON-REUSEABLE SOILS

- A. The Contractor shall be responsible to remove all non-useable soils from the project site except as noted below.
- B. Topsoil and earth, etc. shall be removed from all areas within the limits of work to its full depth. Soil that will not be reused as part of construction shall become the property of the Contractor who shall be responsible for complete removal from the site in a legal manner.

Add Section 31 10 00, paragraph 3.14 as follows:

- 3.14 Dewatering and Handling of Construction Water
 - A. The Contractor is responsible for all dewatering and associated handling activities related to this Work.
 - B. If dewatering is required the Contractor shall comply with all Federal, State and Local regulations related to quantity and quality of generated water.
- 3. Add Section 31 23 19 Dewatering. New Section consisting of 2 pages is attached to this document.

Drawing

- 1. Drawings are revised to include the attached detail consisting of one page entitled, "Steel Pipe Bollard"
- 2. Drawings are revised to include the attached two-sheet drawing entitled;

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"Handhole – Round – Precast Concrete 36" Diameter x 36" High" SPC H-020 6
"Pad – Precast Concrete – Transformer 167 KVA Maximum – 42" x 48" SPC P-009 11.
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- 3. Drawing C2-1 is revised and replaced in its entirety with the attached drawing.
- 4. Drawing C2-2 is revised and replaced in its entirety with the attached drawing.
- 5. New Drawing C4-8 is issued and attached to this Addenda.

DR. CLARENCE WELTI, P.E., P.C.

GEOTECHNICAL ENGINEERING

227 Williams Street · P.O. Box 397 Glastonbury, CT 06033-0397

(860) 633-4623 / FAX (860) 657-2514

December 10, 2014

Mr. Glen S. Yeakel, AIA Friar Associates 281 Farmington Avenue Farmington, CT 06032

Re: Geotechnical Study for Proposed Synthetic Field, Light Poles and School Board Mill Ridge Schools, Danbury, CT

Dear Glen:

- 1.0 Herewith are the data from the test borings taken at the above referenced site. Four borings were taken along the edge of the proposed field, four borings at proposed light pole locations, and one boring at a proposed score board. The boring locations are shown on the attached plan. The borings were drilled by Clarence Welti Associates, Inc. and sampling was conducted by this firm solely to obtain indications of subsurface conditions as part of a geotechnical exploration program. No services were performed to evaluate subsurface environmental conditions. Six grain size gradation and water content tests were performed on soils samples taken from the borings. The results of those tests are included with the boring logs.
- 2.0 The **Subject Project** will include the construction of a new synthetic field, installation of 4 new light poles and installation of a new Score Board. A proposed grading plan was not available at the time of this report. The proposed field will be located atop an existing grass field. The existing grades in the field range from about Elev.568 to Elev.571.
- 3.0 The **Soils Cross Section** from the borings was generally as follows:

Light Poles (see borings L-1 thru L-4)

Topsoil to 6" to 9"

At boring L-1& L-2 - Probable FILL; fine to medium SAND, some Silt, trace Gravel to 8 to 10 feet, medium compact

Moraine; fine to medium SAND and SILT, trace to little Gravel to 10.5 to 21+ feet, dense

At boring L-3; Decomposed Rock from 10.5 to 21+ feet, very dense

At boring L-4; Auger Refusal on probable bedrock at 13 feet

Note: Boring L-4 was moved about 15 feet south of proposed light pole to avoid a conflict with the existing sewer and gas lines.

Score Board (see boring SB-5)

Topsoil to 11"

FILL; fine to medium SAND, some Silt, trace Gravel to 8 feet, loose to medium compact

Moraine; fine to medium SAND, some Silt, trace Gravel to 16.5+ feet, medium compact

Proposed Synthetic Field (see borings F-6 thru F-9)

Topsoil to 7" to 9"

At boring F-6 & F-7 - FILL; fine to medium SAND, some Silt, trace Gravel to 5 feet, medium compact

At boring F-8- FILL; fine to medium SAND, trace Silt to 3.5 feet, medium compact

Moraine; fine to medium SAND, some Silt, trace Gravel to 8+ feet, medium compact to dense

- 3.1 The **Ground Water**, where evident in the boreholes, was at 6 to 14.5 below the existing grades at the completion of the borings. The natural moraine soils have a low permeability and low voids ratio. These properties allow for a large fluctuation/mounding of the water table to occur in areas which receive a concentrated storm water recharge. The soils below about 6 to 8 feet had water contents close to saturation in all of the borings. The natural soils and existing fills will be sensitive to remolding beneath heavy equipment when wet.
- 4.0 In general the criteria for light pole support is that the foundation capacity would exceed the loads, which might collapse the tower. Movements from strains in the soils should be limited to differential settlement (or lateral movements of less than ½").
- 5.0 The light pole and score board foundation type could be one of the following:
- 1. A mat (spread footing) acting with it own mass to provide the required resistance to overturning
- 2. A caisson or drilled pier type foundation

3. Direct burial type pole installed in cased hole

- 5.1 In alternate (1) the weight of the mat would provide the required resistance to over turning. The foundations can be placed on the natural inorganic soils at least 3.5 feet below the finished grades or on a controlled fill placed after the removal of any existing fills and organic soils. The controlled fill should conform to section 6.0 below and should extend outside of foundations for a distance equal to at least the depth of fill beneath foundations. The allowable loading on the natural inorganic soils at 3.5 feet below grade or on the controlled fill can be 2 Tons/sf. There should be a minimum 6" layer of 3/8" crushed stone beneath the mat on the natural soils and as an initial layer beneath controlled fills where on a wet sub grade.
- 5.2 In summary the following soil properties and design values would apply to alternate 1.

Soil Property/Parameter	Value
Soil Unit Weight (Backfill) above water table	125 pcf
Soil Unit Weight (Natural)	130 pcf
Soil Unit Weight Submerged	68 pcf
Angle of Internal Friction (φ)	34°
Active Pressure Coefficient	0.28
Cohesion	0
Sliding Coefficient	0.5
Frost Protection Depth (by code)	3.5 feet
Allowable Bearing Pressure on natural soils or controlled fill	2 Tons/sf

5.3 **Alternate 2** would be a caisson foundation. The caisson shall have a minimum embedment depth of 12 feet. The actual depth is to be determined by the designer to provide the required resistance to overturning forces as well as maintaining the allowable lateral deflection**. The following is a summary of design parameters for the caisson foundation:

stratum depth	Total Unit Weight (pcf)	Effective (submerged) Unit Weight (pcf)	Friction Angle degrees	Soil Modulus Parameter, k - above groundwater (pci) *	Soil Modulus Parameter, k - below groundwater (pci) *	Allowable Bearing Pressure (TSF)
Boring Number L-1 - bottom of	f boring at 20.3', ground	lwater assumed at about	10' based on soil s	ample water contents		
0 to 8'+ FILL; fine to medium SAND, some some Silt, trace Gravel	125	63	30	90	60	-

8.0 to 20.3'+ fine to medium SAND, some Silt, trace Gravel	125	63	34	225	125	4.0						
Boring Number L-2 - bottom of boring at 21.5', groundwater assumed at about 10' based on soil sample water contents												
0 to 8.0' fine to medium SAND, some Silt, trace Gravel	125	63	30	90	60	-						
8.0' to 21.5' fine to medium SAND, some Silt, trace Gravel	125	63	34	225	125	4.0						
Boring Number L-3 - bottom of bo	oring (auger refusal)	at 13.0', groundwater at 1	0.0' at completion	of boring								
0 to 13' fine to medium SAND, some Silt, trace Gravel	125	63	34	225	125	4.0						
Boring Number L-4 - bottom of bo	oring at 21.5', ground	lwater assumed at about	10' based on soil sa	ample water contents								
0 to 10.5' fine to coarse SAND, some Gravel, Silt	135	73	36	225	125	4.0						
10.5' to 21.5'+ Decomposed Rock	140	78	36	225	125	6.0						

^{*} This value should be used to evaluate lateral deflection at top of caisson. Coefficient of Subgrade Reaction, k=k₁ (Z/B), where Z is depth and B is the pile width

The lateral deflection can be analyzed from Lpile Program or from a empirical formulas in Drilled Pier Foundations; Woodward Gardener Greer; Mcgraw Hill 1972. The soils to about 3 feet below the finished grades should be ignored in the calculating the lateral resistance.

- 5.4 Alternate 3, the direct burial poles, can be designed using the parameters in the table in section 5.3 above. It is assumed that buried section of the pole would have a diameter of about 24" and will be installed in a minimum 36" diameter hole. The required minimum burial depth to maintain lateral movements within the above criteria would be in the range of 12 to 16 feet below grade. The required size and depths of the drilled shafts and direct burial poles must be determined from a lateral analysis based on the design loading and performance criteria. The annular space between the pole and the casing should be filled with concrete.
- 6.0 The foundations for the bleachers can be with spread footings. The recommendations and design parameters from sections 5.1 and 5.2 above would apply to the those structures.
- 7.0 Regarding Controlled Fill beneath foundations and backfill of foundations, the material shall conform to the following gradation, CTDOT M.02.02, or be 3/8" crushed stone:

Percent Passing	S	ieve Size	

^{**} Typically this value would be about ½"

100	3.5"
50 - 100	3/4"
25 - 75	No.4

The fraction, passing the No.4 sieve shall have less than 15%, passing the No. 200 sieve.

The on site soils will generally not conform to the above gradation.

All controlled fill and backfill must be compacted to at least 95% of modified optimum density in accordance with ASTM D-1557.

- 8.0 Site Retaining Walls (if any) can be designed using active pressure. The active pressure coefficient would be 0.28 for walls with level backfill and 0.45 for walls with a 2H:1V back slope. The foundations for site retaining walls can be on the natural soils or on a controlled fill placed after the removal of any existing fills, topsoil and organic soils. The recommendations from sections 5.1 and 5.2 above would also apply for site retaining walls. There should be a foundation drain behind all retaining walls.
- 9.0 The **new athletic field** is understood to be a synthetic turf field. The general criteria for the layers directly under the field are usually part of a design build section. Apart from this section the sub grades must be capable of supporting the construction equipment without rutting. A second requirement would be a total section of least 18" of non-frost susceptible soils. The soils beneath the existing topsoil will not meet the requirement for non-frost susceptible materials and may also not be stable beneath construction equipment when wet. If the sub grades can be proof rolled without significant rutting or movements and there is at least 18" of non-frost susceptible material beneath the field the existing fills and natural soils could be left in place. If the sub grades are unstable there would be a requirement to place an initial layer 12"± layer of 3/4" crushed stone atop a geotextile to support the equipment required to build the field. Dependent on the soil subgrade conditions, the crushed stone may be part of or in addition to the 18" section cited above.
- 10.0 The soils at the subject site are generally in OSHA Class C which would require excavations that are in excess of 5 feet to have slopes which are less than 34° i.e. 1.5H to 1.0V.
- 11.0 This report has been prepared for specific application to the subject project in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made. In the event that any changes in the nature, design and location of structures are planned, the conclusions and recommendations contained in this report should not be considered valid unless the changes are reviewed and conclusions of this report modified or verified in writing.

The analyses and recommendations submitted in this report are based in part upon data obtained from referenced explorations. The extent of variations between explorations may not become evident until construction. If variations then appear evident, it will be necessary to re-evaluate the recommendations of this report.

May White

Dr. Clarence Welti, P.E., P.C., should perform a general review of the final design and specifications in order that geotechnical design recommendations may be properly interpreted and implemented as they were intended.

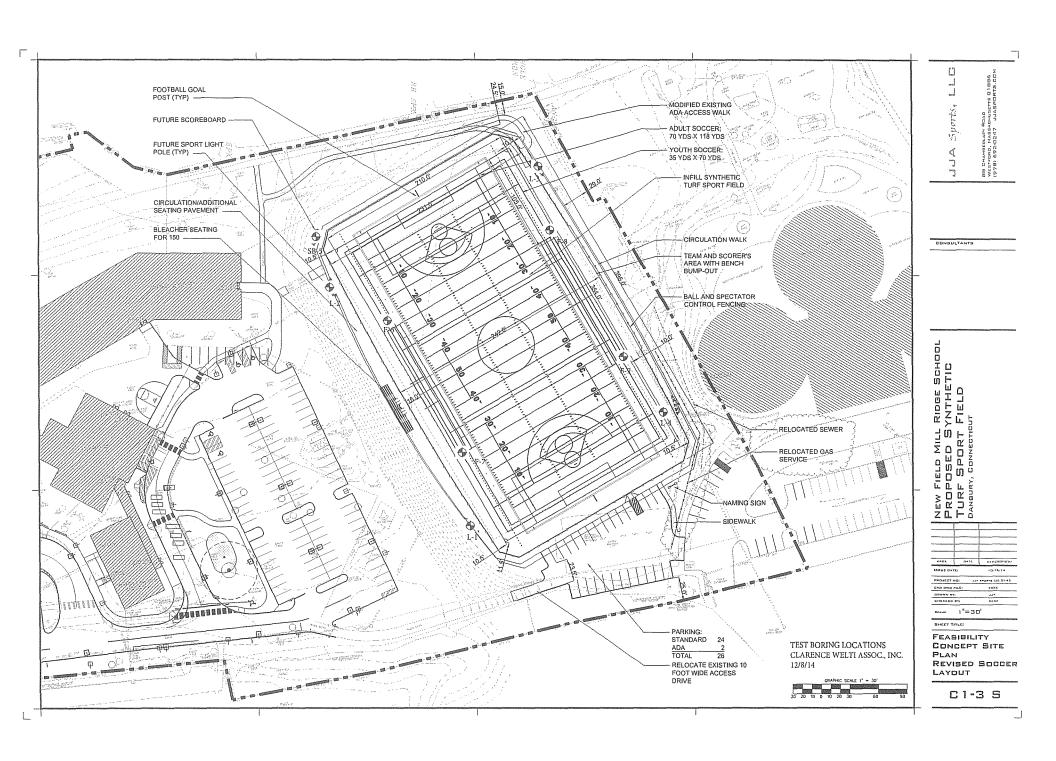
If you have any questions please call me.

Very truly yours,

Max Welti, P.E.

Clarence Welti Ph.D., P. E. President, Dr. Clarence Welti P.E.; P.C.

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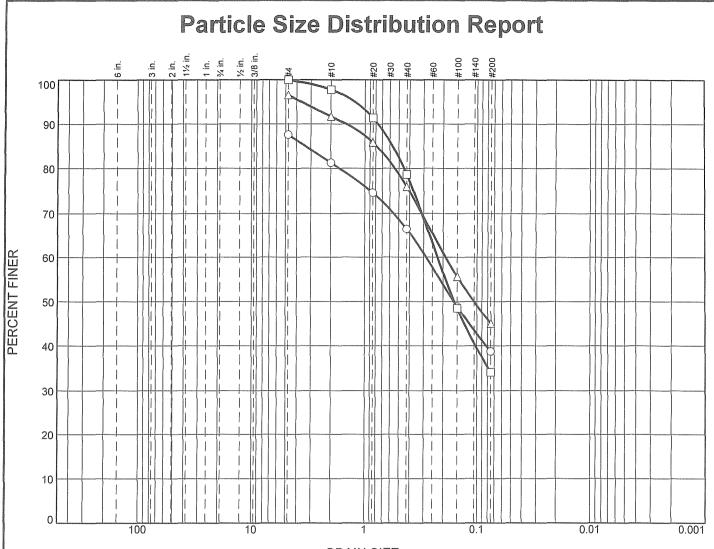
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TYPE		HSA 2.75"		SS	1		LINE & STA.	GROUND WATER OBSI		START DATE	12/8/14
SIZE I.E		3.75"		1.375"	 		N. COORDINATE	AT none FT. AFTER	0 Hours	DATE	
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DEPTH	NO.	BLOWS/6		PTH A			STRATUM	DESCRIPTION + REMARKS			ELEV.
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		AUGER	CASING	SAMPLER	CORE		OFFSET	SURFACE ELEV.	HOLE		F-8
TYPE		HSA		SS			LINE & STA.	GROUND WATER OBSE			***************************************
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		3-3-0-17	2.00	-4.00						—∖ 3.£	
	3	10-16-26-2	1 4.00'	-6.00'	-	-	REY/BR.FINE-MED.SAND, SC	OME SILT, TRACE GR	AVEL	4.5	
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CLARENCE WELTI ASSOC., INC. P.O. BOX 397 GLASTONBURY, CONN 06033		CLIE	CLIENT			PROJECT NAME PROPOSED SYNTHETIC TURF ATHLETIC FILL LOCATION									
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OTHER DESIGNATION OF THE PERSON				G	<u>RAIN SIZE -</u>	· mm.					
	% +3"		% Gra	% Gravel		% Sand	t	% Fines			
			Coarse	Fine	Coarse	Medium	Fine	Silt		Clay	
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Δ					4.8	15.8	30.9		45.0		
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			0.5668	0.2247	0.15	92					
			0.7856	0.1902	0.10	61					

USCS **AASHTO Material Description**

Project No. **Client: FRIAR ASSOCIATES**

Project: PROPOSED SYNTHETIC TURF ATHLETIC FIELD

O Source of Sample: F-9 **Depth:** 0.60 ☐ Source of Sample: L-1

△ Source of Sample: L-3

Depth: 4.0

Depth: 2.0 Sample Number: 2

CLARENCE WELTI ASSOCIATES, INC.

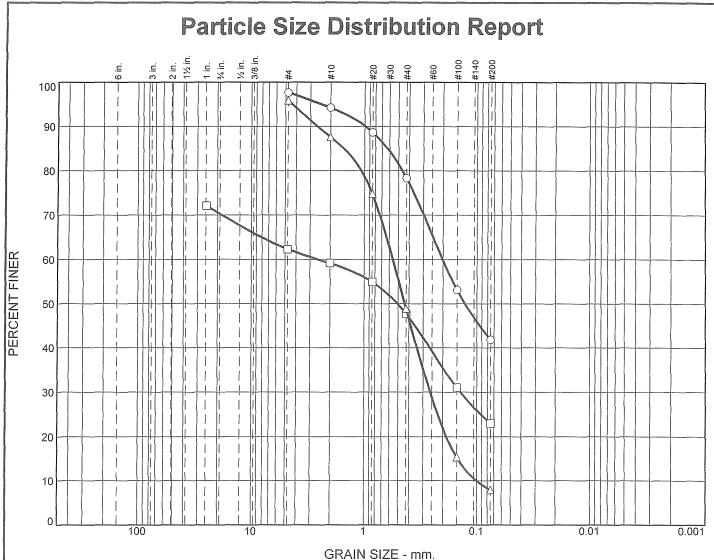
Remarks:

OWATER CONTENT = 10.6%

□ WATER CONTENT = 16.3%

△WATER CONTENT = 11.1%

Figure



					G	RAIN SIZE	<u>- mn</u>	<u>n</u>			
	% +3" % G			avel % Sand				% Fines			
	70 T3		Coarse	Fine	Coarse	Medium		Fine	Silt		Clay
					3.5	15.9		36.5		41.8	
			8.0	3.2	11.4		24.8	22.9			
					8.3	38.8		40.9		7.9	
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Project No.

Client: FRIAR ASSOCIATES

Project: PROPOSED SYNTHETIC TURF ATHLETIC FIELD

O Source of Sample: F-6

Depth: 2.0

Sample Number: 2

□ Source of Sample: F-7

Depth: 2.0

Sample Number: 2

△ Source of Sample: F-8

Depth: 0.60

CLARENCE WELTI ASSOCIATES, INC.

Remarks:

○ WATER CONTENT = 14.3%

□ WATER CONTENT = 7.8%

△ WATER CONTENT = 16.3%

Figure

		PROPOSAL	P-1
TO:	Purchasing Agent, City	y of Danbury, Connecticut	
FOR:	Bid No. 01-14-15-05 Project No. 14-28	Westside Middle School Academy Multipurpose Field	
MADE	EBY:		
		(Firm Name)	
work, hereby equipm or propactor furnish	g thereto, the undersigned and that he understands offers to furnish and in hent, labor, overhead, proper for or incidental to, to ance with the Contract ed from time to time d	ompliance with your Invitation To Bid and the Inced states that he has examined the Contract Documes the purport and magnitude of the work intended stall the Work. In his or her Bid they shall include rofit, bond, insurance, warrantees and other facilities the proper construction of the work, and to construct Documents and such detailed directions, plans, are uring the progress of construction by the Engineer all incidental work, viz.:	nents and the site of the d, and the undersigned e all materials, supplies, es and things necessary et the said work in strict and drawings as may be
turf du buildin and de scorebe walkwa bleache	al fiber field system wing permits, installing and emolition, removals, depard, synthetic turf base ays, fencing, ball controllers seating, tree planting	f upgrading the existing natural turf grass sport field th venue support elements. More specifically the W I maintaining erosion and sedimentation control me isposals, installation of stormdrainage, electric system, resilient padding and liner system, concrete of netting, a parking lot, accessible walkways, a re gs, and other site improvements at the campus on W specifically described in the Contract Documents.	Vork includes obtaining easures, site preparation service, field lighting, e and asphaltic concrete ecognition naming sign,
_	-	base bid and (4) four alternates to the project. A denined in the Bid Documents.	escription of the base
provide include	ed to the Contractor for i	omponent of the project will be purchased separately installation by the selected synthetic turf vendor. The ion, supervision and management required to support	ne Contractor shall
specifi 2015.		grees to commence work under this contract on or Proceed" of the Owner, and to fully complete the	
The Luetc.:	ump Sum Price for all W	Vord required in the Bid Documents inclusive of all	fees, bonds, insurance,
(T T)		- av 1)	
(In Fig	ures)	(Words)	

Westside Middle School Academy Multipurpose Field DANBURY, CT

P-2

ALTERNATES

Alternates are amounts proposed by Bidders and stated on the Bid Form for certain construction activities defined in the Bidding Requirements that may be added to or deducted from Base Bid amount if the Owner decides to accept a corresponding change in either the amount of construction to be completed, or in the products, materials, equipment, systems or installation methods described in the Contract Documents.

The Alternate pricing shall include all related supervision, coordination, schedule time, Work and modifying and or adjustments to adjacent Work as necessary to ensure that Work affected by each accepted Alternate is complete and fully integrated into the Project. Costs listed for each Add Alternate shall include costs of related coordination, modification, or adjustment.

Alternates are generally listed below and more fully described in SECTION 01 11 00 SUMMARY OF WORK and SECTION 01 23 00 ALTERNATES.

Alternate No	Abbreviated Description	Cost
Alternate No. 1	Slab on-grade aluminum bleacher system with accessible seating cutouts.	Cost in Dollars (Figures): Cost in Dollars (Words)
Alternate No. 2	Multisport Digital Wireless Scoreboard	Cost in Dollars (Figures): Cost in Dollars (Words)
Alternate No. 3	Sport/Security Lighting System	Cost in Dollars (Figures): Cost in Dollars (Words)
Alternate No 4	Integral synthetic turf anchor trench drain system	Cost in Dollars (Figures): Cost in Dollars (Words)

Project No. 2014-083A Print Date: 3/9/2015

ADDITIONAL CONDITONS OF THE BID

P - 3

The bidder further declares that he/she is/they are the only person/persons interested in the Proposals and that it is made without any connection with any other person or persons making proposals for the same work and that it is in all respects fair and without collusion or fraud.

And he/she/they does/do hereby agree that if this Proposal is accepted, he/she/they will execute and deliver bond in a penalty equal to the amount of the Contract to be approved by the City of Danbury, to construct the work at the price and upon the terms proposed according to the drawings and specifications filed with the City.

Accomp	anying this Proposal, under separate cover, is a Bid Bond, Cashier's Check, or Certified
	Dollars
<u>(\$</u>) payable to the City. In case this Proposal is accepted by the City, and the
(4	Il fail to execute the Contract with, and to give a bond to the City, according to the
Information for	Bidders, then the said Bid Bond, Cashier's Check, or Certified Check shall become the
property of the	City; otherwise it shall be returned to the undersigned.

If written notice of the acceptance of this bid is mailed, telegraphed, or delivered to the undersigned within ninety (90) days after the date of opening of the bids, or any time thereafter before this bid is withdrawn, the undersigned shall, within fifteen (15) days after such date of mailing, telegraphing or delivering of such notice, deliver to the City the Performance and Payment Bond, Insurance Certificate and Corporate Resolution Form. The Contract will be executed within ten (10) days of receipt of all required information.

Project No. 2014-083A Westside Middle School .
Print Date: 3/9/2015 DAN

UNIT PRICES

P - 4

Should certain additional work be required, or should the quantities of certain classes of work be increased or decreased from those on which the General Bid is to be based by order or approval of the Architect, the undersigned agrees that the credit and deduct unit prices provided by the contractor in the table below will be the basis of payment to him or credit to the Awarding Authority for such addition, increase or decrease in the Work for the duration of the Project. Unit Prices given shall represent the exact new amount per unit to be paid the Contractor (in the case of additions or increases) or to be refunded the Awarding Authority (in the case of decreases). No additional adjustment will be allowed for overhead, profit, bonds, insurance, compensation insurance or other direct or indirect expenses of Contractor or Subcontractors.

Schedule Of Unit Prices – REFER TO TECHNICAL SPECECTIFICATIONS FOR ADDITIONAL INFORMATION

Unit		Add	Deduct
1.	4 inch PVC conduit, complete in-place, including excavation, sand bedding and backfill, and cover at 24 inch-depth, per linear foot		
2.	2 inch PVC conduit, complete in-place, including excavation, sand bedding and backfill, and cover at 24 inch-depth, per linear foot		
3.	12 inch diameter HDPE drain pipe, complete in-place, including excavation, bedding and backfill, including 3 ft. cover, per linear foot.		
4.	Electrical boxes, each will be 30 inch x 18 inch x 18 inch deep Quasite or approved equal complete in-place, per each.		
5.	4 inch diameter perforated PVC sub drain pipe, complete in-place including excavation, fabric, bedding, drainage stone and backfill, including 3 foot cover, per linear foot.		
6.	4 inch diameter PVC sub drain pipe including excavation, bedding and backfill, including 18 inch cover, per linear foot.		
7.	45 inch height chain link fence, complete inplace, including excavation and concrete footings, per linear foot.		

See Item 8. On next page.

Project No. 2014-083A

Print Date: 3/9/2015

P - 5

8. Provide a standard 36" round transformer concrete vault (see attached) with a top 3'x3' hole; CL&P will install a switch and enclosure. The location of the vault shall be approximately 3' to 5' from the existing CL&P service pole. Final location shall be determined by CL&P. The contractor shall install the vault, schedule 40 - 4" conduit and sweep onto the pole. 4" schedule 40 conduit from the switch vault to the transformer vault entering on the left side. Schedule 40 4" conduit then will have to be run from the right side of the transformer vault to the meter pedestal location. This Unit Price shall include three additional steel bollards. The unit price shall include all excavations, trenching, concrete, backfilling, grading, topsoil and reseeding. The unit price shall include all overhead, profit, fees, insurance, and bond costs.

The above unit prices shall include all labor, materials, bailing, shoring, removal, overhead, profit, fees, bonds, insurance, etc., to cover the finished in-place work of the several kinds called for. No additional mark-up will be applied to unit price related additions or deductions to the contract price.

Project No. 2014-083A Westside Middle S Print Date: 3/9/2015

QUALIFICATIONS, EXPERIENCE AND REFERENCES

P-6

By submitting a bid the Bidder acknowledges the importance of being qualified to perform the specialty type of work contained in the Bid Documents and further represents that he/she has the requisite experience necessary to complete the project in a successful manor for the Owner. In addition the Proposer further acknowledges that the he/she has successful experience in performing higher level quality control and on-going asbuilt work to assure that work of this nature is completed within regulated, recommended and required levels of quality, and that the nature of accuracies and requirements of synthetic turf sports field systems are such that the quality control requirements exceed typical site construction and that these additional efforts outlined in the Documents are included within the submitted Bid.

By submitting a Bid and Signing below the Bidder acknowledges and accepts all conditions of the Bid Documents, and further acknowledges and accepts that the Qualification and Experience requirements for the Bidder (General Site Contractor), key Subcontractors and Vendors have been met.

Signed	Title		
Corporation-Partne	rship-Owner	Incorporated in the State of	

010000-20 Project No. 2014-083A Print Date: 3/9/2015

	FIRM INFORMATION	P - 7
Firm Name **		
Address		
Telephone Number		
By (signature)		
Signed by (printed or typed)		
Title		
Email		
Date		
* Cross out words which do not ap	oply.	
** If a corporation, give the State laws of"	e of Incorporation, using the phrase "a corporation organized und	ler the
	artners, using also the phrase "Co-partners trading and doing bu	ısiness
	me, give individual name, using also the phrase "an individual style of".	doing

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LISTING OF PARTNERS OR PRINCIPALS

Project No. 2014-083A Westside Middle School Academy Multipurpose Field 010000-22

Print Date: 3/9/2015 DANBURY, CT

SECTION 31 23 19 - DEWATERING

PART 1 - GENERAL

1.1 **SUMMARY**

A. Section includes construction dewatering.

1.2 PERFORMANCE REQUIREMENTS

A. Dewatering Performance: Design, furnish, install, test, operate, monitor, and maintain dewatering system of sufficient scope, size, and capacity to control hydrostatic pressures and to lower, control, remove, and dispose of ground water and permit excavation and construction to proceed on dry, stable subgrades.

1.3 **SUBMITTALS**

Shop Drawings: For dewatering system. Show arrangement, locations, and details of wells and Α. well points; locations of risers, headers, filters, pumps, power units, discharge lines, piezometers, and flow-measuring devices; and means of discharge, control of sediment, and disposal of water.

QUALITY ASSURANCE 1.4

- Regulatory Requirements: Comply with governing EPA notification regulations before A. beginning dewatering. Comply with hauling and disposal regulations of authorities having jurisdiction.
- B. Preinstallation Conference: Conduct conference at [Project site] < Insert location>.

1.5 PROJECT CONDITIONS

- A. Survey Work: Engage a qualified land surveyor or professional engineer to survey adjacent existing buildings, structures, and site improvements, establishing exact elevations at fixed points to act as benchmarks. Clearly identify benchmarks and record existing elevations.
 - 1. During dewatering, regularly resurvey benchmarks, maintaining an accurate log of surveyed elevations for comparison with original elevations. Promptly notify Architect if changes in elevations occur or if cracks, sags, or other damage is evident in adjacent construction.

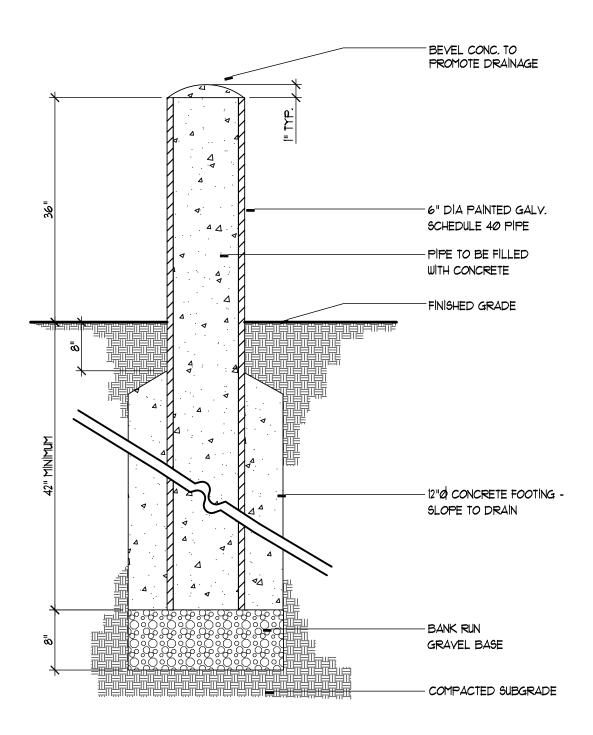
PART 2 - PRODUCTS (Not Used)

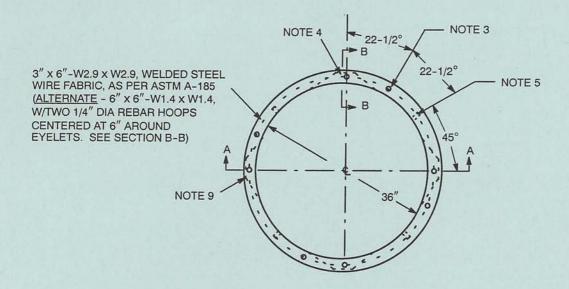
PART 3 - EXECUTION

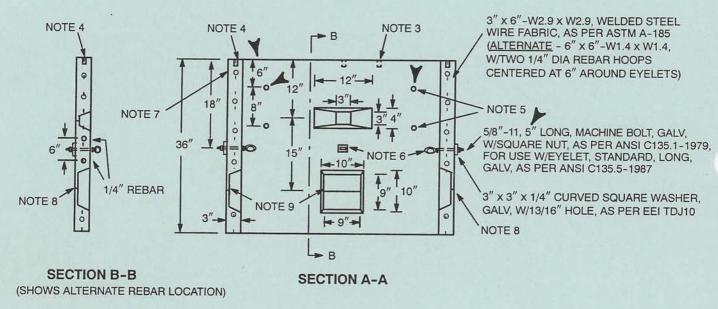
3.1 **INSTALLATION**

- A. Provide temporary grading to facilitate dewatering and control of surface water.
- B. Monitor dewatering systems continuously.
- C. Protect and maintain temporary erosion and sedimentation controls, which are specified in during dewatering operations. Discharges from dewatering operations into the on-site sedimentation basins shall be via additional treatment as designed by the Contractor based on dewatering proposed rates.
- D. Install dewatering system utilizing wells, well points, or similar methods complete with pump equipment, standby power and pumps, filter material gradation, valves, appurtenances, water disposal, and surface-water controls.
 - Space well points or wells at intervals required to provide sufficient dewatering. 1.
 - Use filters or other means to prevent pumping of fine sands or silts from the subsurface. 2.
- E. Before excavating below ground-water level, place system into operation to lower water to specified levels. Operate system continuously until drains, sewers, and structures have been constructed and fill materials have been placed or until dewatering is no longer required.
- Provide an adequate system to lower and control ground water to permit excavation, F. construction of structures, and placement of fill materials on dry subgrades. Install sufficient dewatering equipment to drain water-bearing strata above and below bottom of foundations, drains, sewers, and other excavations.
 - Do not permit open-sump pumping that leads to loss of fines, soil piping, subgrade 1. softening, and slope instability.
- Reduce hydrostatic head in water-bearing strata below subgrade elevations of foundations, G. drains, sewers, and other excavations.
 - Maintain piezometric water level a minimum of 24 inches below surface of excavation.
- H. Provide standby equipment on site, installed and available for immediate operation, to maintain dewatering on continuous basis if any part of system becomes inadequate or fails. If dewatering requirements are not satisfied due to inadequacy or failure of dewatering system, restore damaged structures and foundation soils at no additional expense to Owner.
 - 1. Remove dewatering system from Project site on completion of dewatering. Plug or fill well holes with sand or cut off and cap wells a minimum of 36 inches (900 mm) below overlying construction.

END OF SECTION 02240



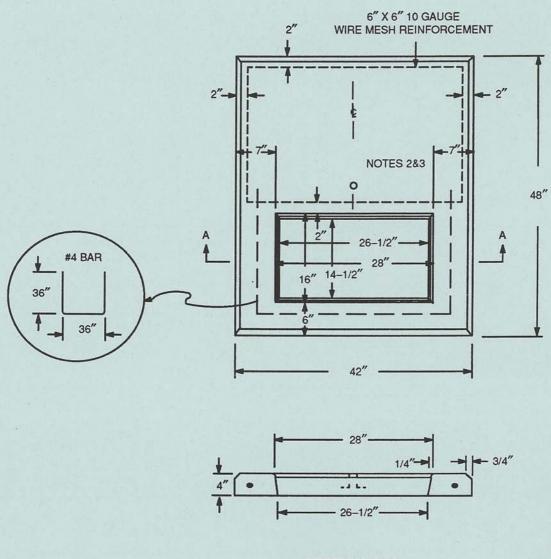




Notes

- 1. Concrete and concrete design shall be in accordance with ACI 318-1995.
- 2. Concrete strength 5000 psi at 28 days. Air entrainment 6 to 8 percent.
- 3. Four zinc alloy inserts 3/8" 16 x 1-3/8" equally spaced on 38-1/2" bolt circle.
- ▲4. Four zinc alloy inserts 5/8" 11 x 1-11/16" equally spaced on 38-1/2" bolt circle.
 - 5. Eight zinc alloy inserts 3/8" 16 x 1-3/8".
 - 6. Four eyelets, for use as pulling points, shall be installed adjacent to and centered above knockouts as shown.
 - 7. Manufacturer's identification and month/year when manufactured shall be legibly marked in/on concrete on the outside upper part.
 - 8. Location of each knockout shall be identifiable from exterior of handhole by providing a 3" diameter circle imprinted 1/4" deep into the concrete. Center of the imprinted circle shall be located at center of each knockout.
 - 9. All knockouts shall have a web thickness not to exceed 3/4" ±1/8". Knockouts to be free of reinforcement.
 - 10. The two recesses 12" from top are for use with adjustable lifter tool only.
- 11. Eyelets and zinc alloy inserts not to be used for lifting.

ORIGINAL 5/13/86	HANDHOLE - ROUND - PRECAST CONCRETE 36" DIAMETER x 36" HIGH					
APPROVED						
6/9/00 RYD-	NORTHEAST UTILITIES	MATERIAL SPECIFICATION	SPC H-020	6		

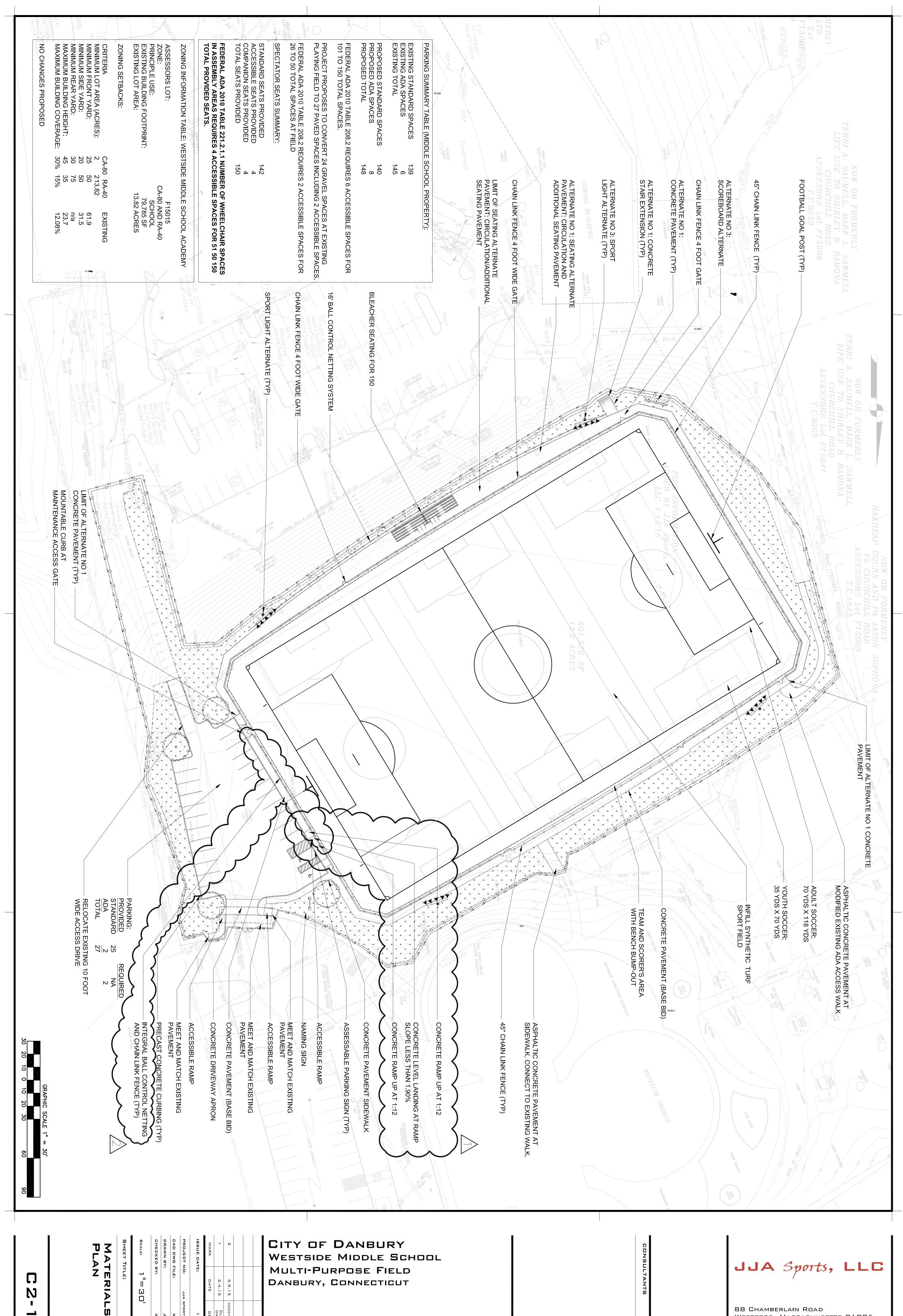


SECTION A-A

NOTES

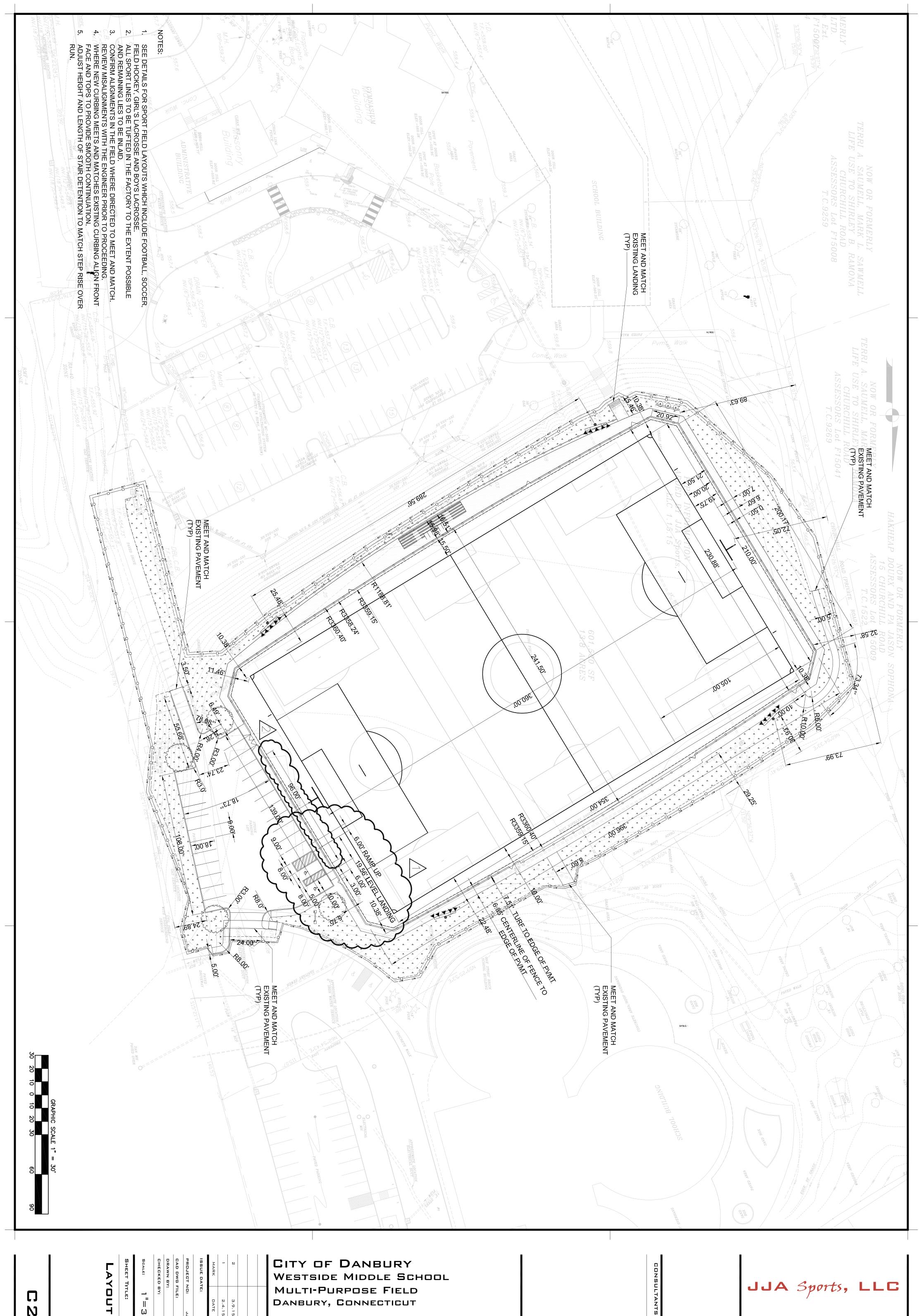
- 1. Final 28 day concrete strength to be 5000 psi.
- 2. A 3/4 inch coil loop insert (Dayton F63) with an insert locator plug (Dayton T21) for lifting the pad at the center of gravity with a swivel plate.
- 3. Manufacturer's identification and month/year when manufactured shall be legibly marked in/on concrete in the top near the center.
- 4. Concrete and concrete design shall be in accordance with ACI 318-1986.

ORIGINAL	PAD – PRECAST CONCRETE – TRANSFORMER						
4/23/70	TAD THEOROT CONCILIE - HIANOT CHIMEN						
APPROVED	167 KVA MAXIMUM – 42" X 48" X 4"						
8/1/91 FStruens	NORTHEAST UTILITIES	MATERIAL SPECIFICATION	SPC P-009	11			



88 CHAMBERLAIN ROAD

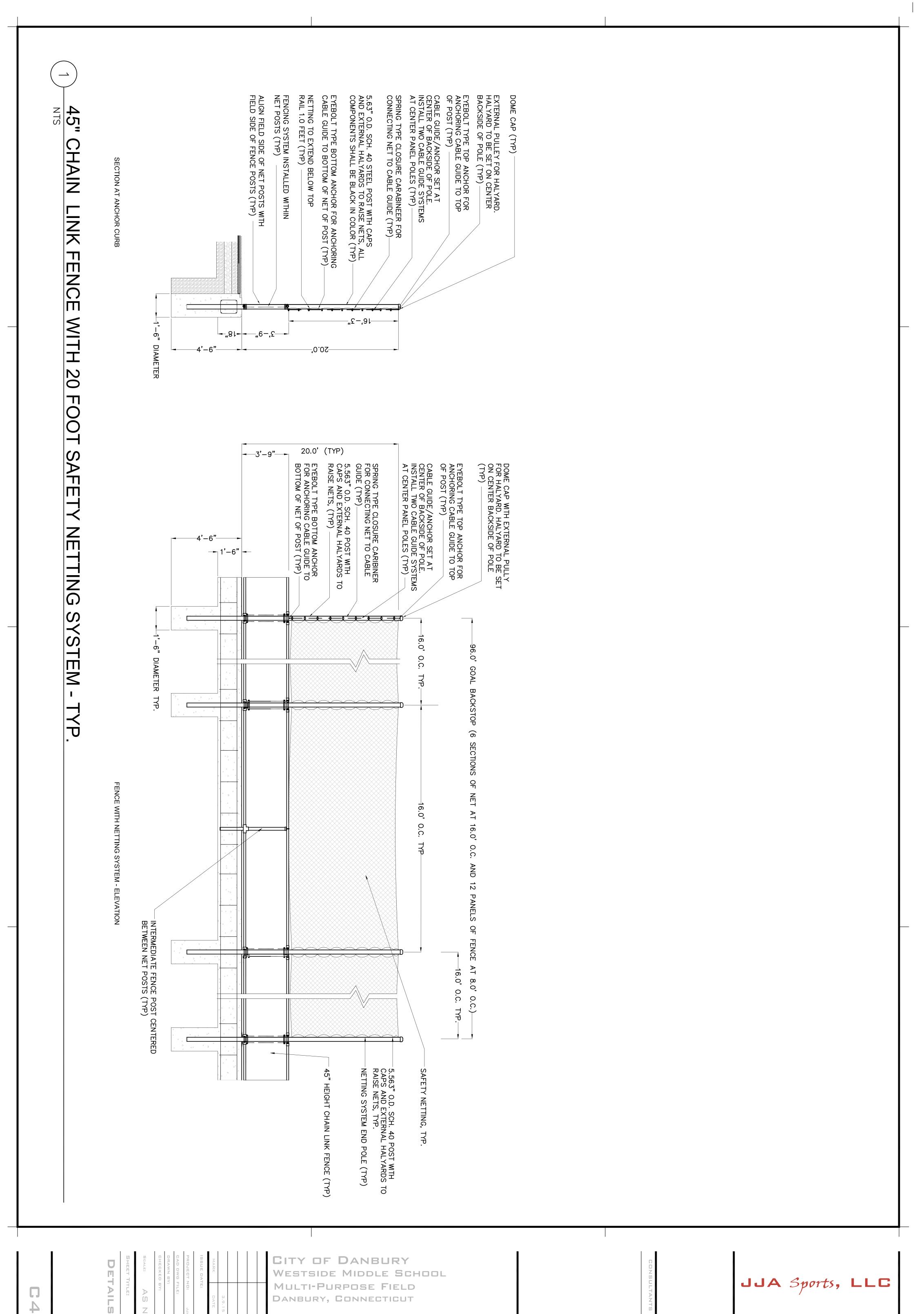
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NOTED